

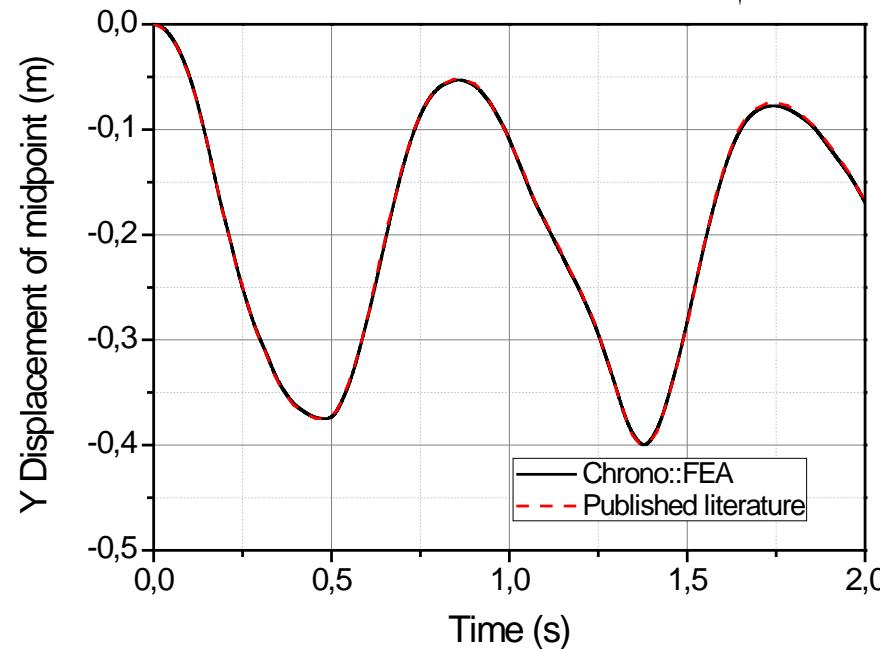
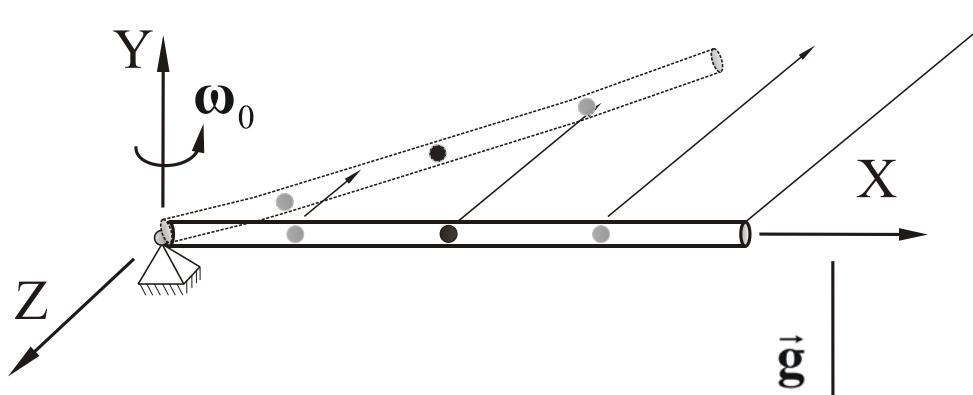


Chrono::FEA

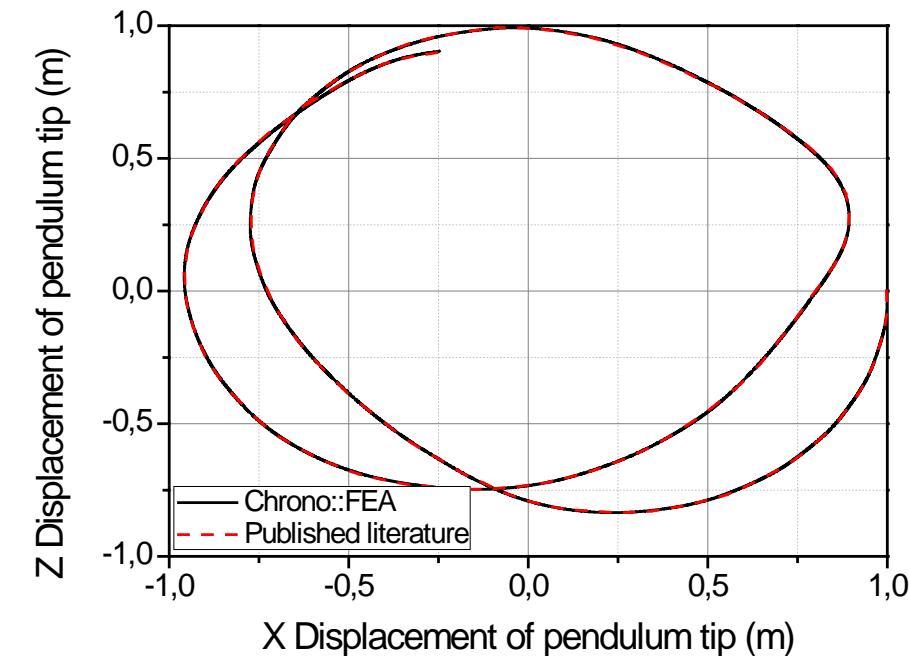
Validation



ANCF Cable

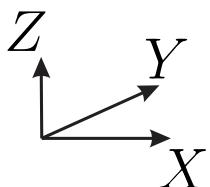
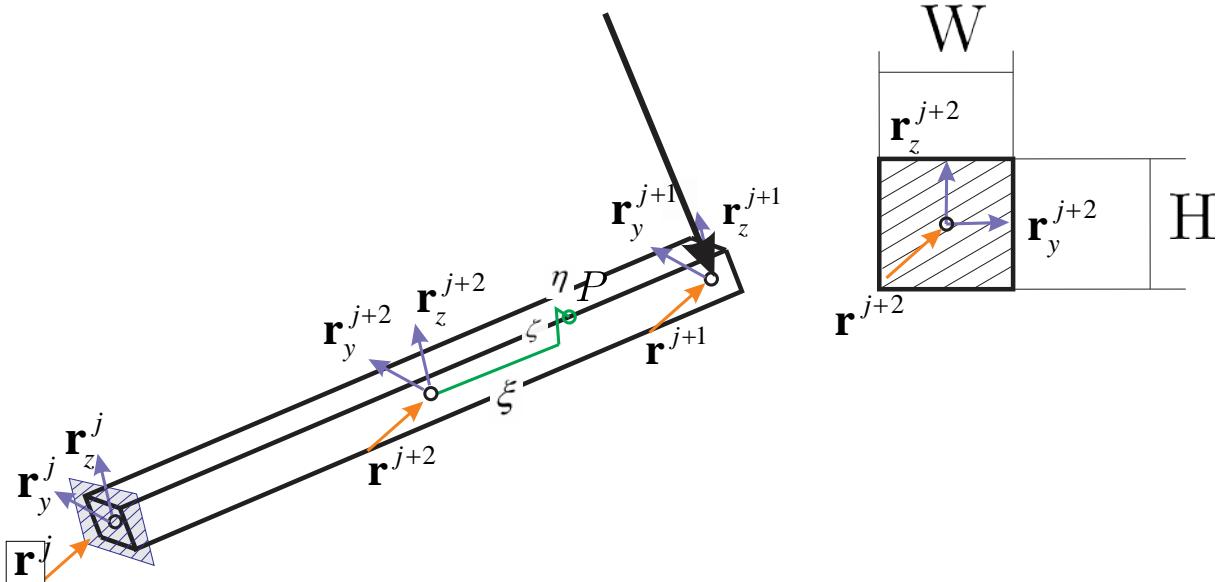


ANCF cable elements validated against published literature (see unit test `test_ANCF Cable.cpp`)



*Chrono's implementation has been verified against: Gerstmayr and Shabana, 2006, "Analysis of thin beams and cables using the absolute nodal coordinate formulation", Nonlinear Dynamics 45: 109–130

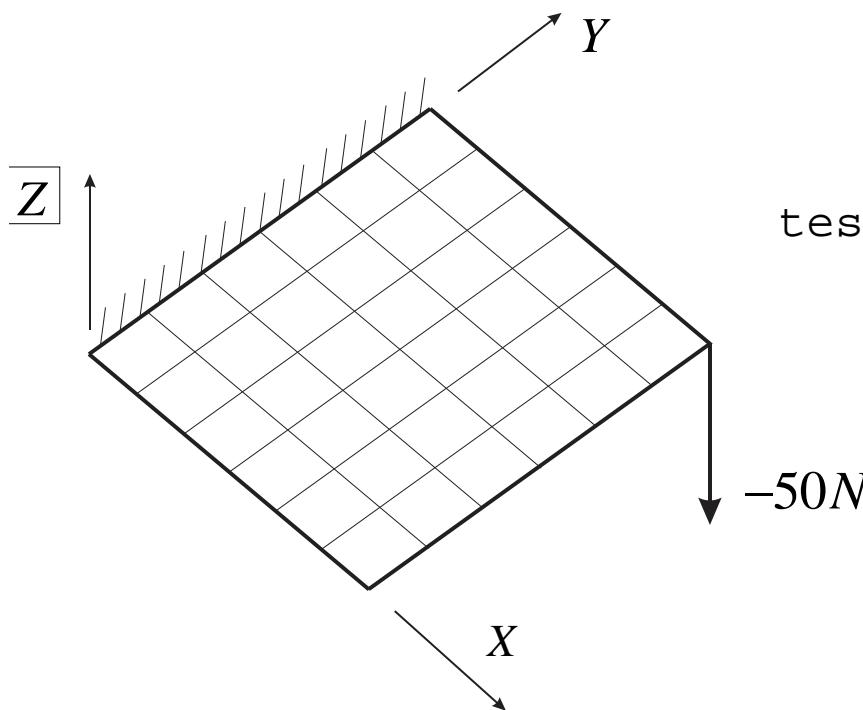
ANCF Beam



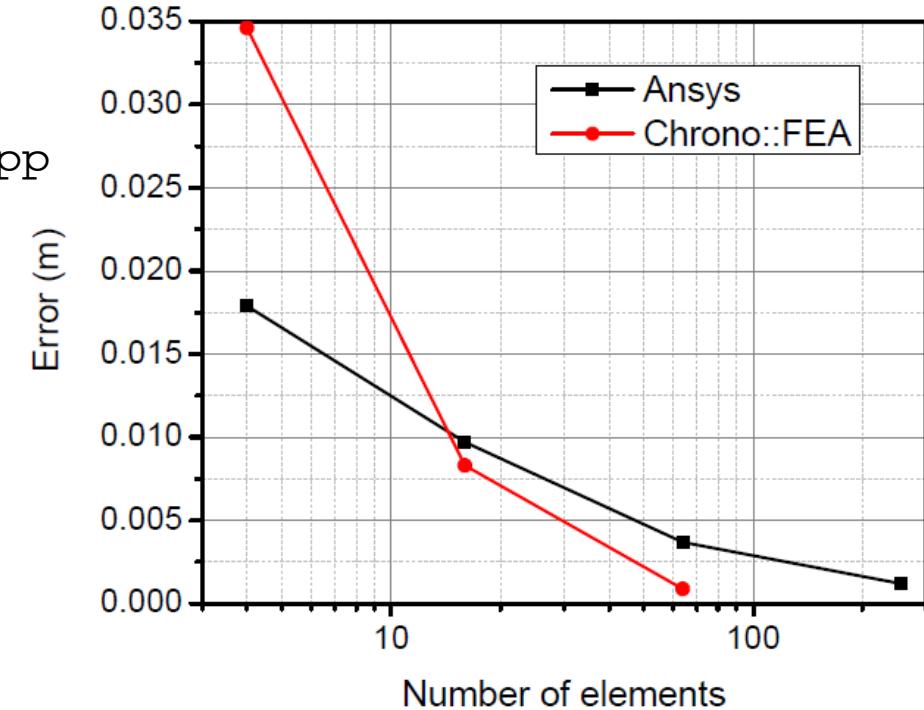
- $H = 0.5\text{m}$; $W = 0.1\text{m}$; $L = 2.0\text{m}$; 4 ANCF finite elements
- $E = 2.07e11 \text{ Pa}$; Poisson ratio = 0.3; k_1, k_2 Timoshenko coefficients
- Force = $-5e5 0.5^3 \text{ N}$
- Results match up to numerical precision with published in the literature: "Structural and continuum mechanics approaches for a 3D shear deformable ANCF beam finite element: Application to static and linearized dynamic examples", Journal of Computational and Nonlinear Dynamics, April 2013, Vol. 8/021004.
- Verified for small and large deformation

ANCF cable elements validated against published literature (see unit test `utest_ANCFBeam.cpp`)

ANCF shell

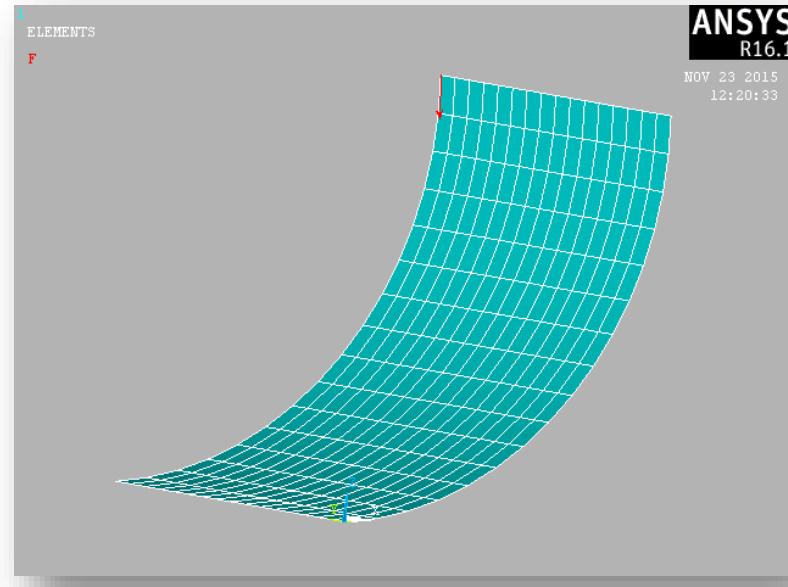


Isotropic
test_ANCFShell_Iso.cpp

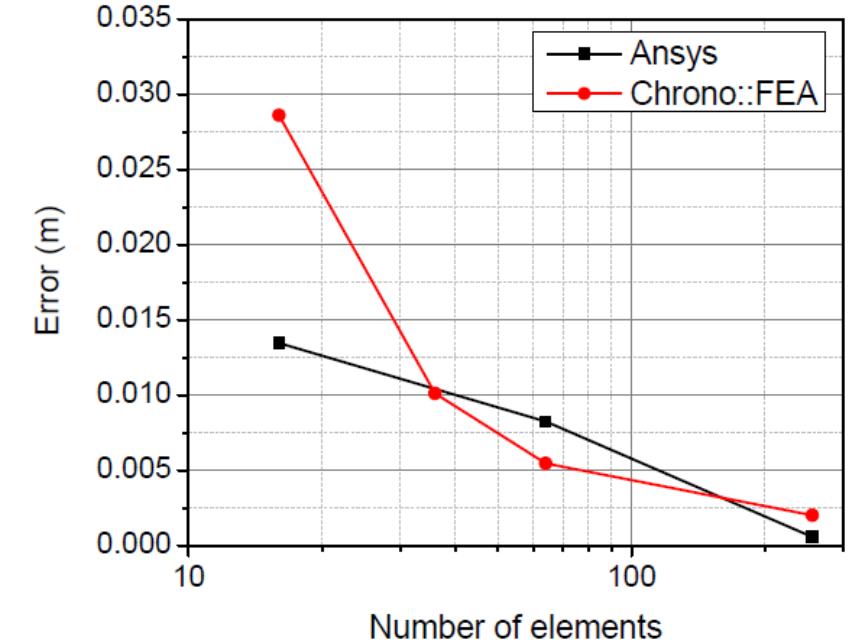


Dimensions	E (MPa)	G (MPa)	Density	Vertical Force	Simulation type	Ansys element	Converged disp
1mx1mx0.01 m	210	80.8	500 kg/m ³	-50N	Dynamic	Shell181 (EAS)	-0.649m

Orthotropic and Composite

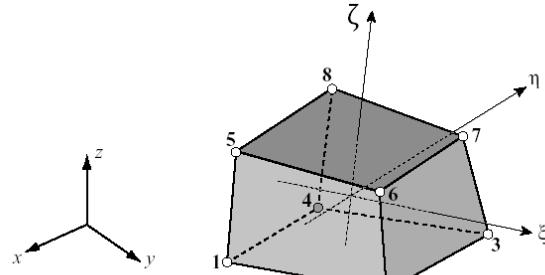


test_ANCFSHELL_Ort.cpp



Dimensions	Ex (MPa)	G (MPa)	Ey=Ez (MPa)	Density	Vertical Force	Simulation type	Number of layers	Thickness of each layer	Fiber angle	Converged disp.
1mx1mx0.01m	200	38.5	100	500 kg/m ³	-10N	Dynamic	2	0.005m	20 degrees	-0.80207m

EAS Brick element



8-noded brick element

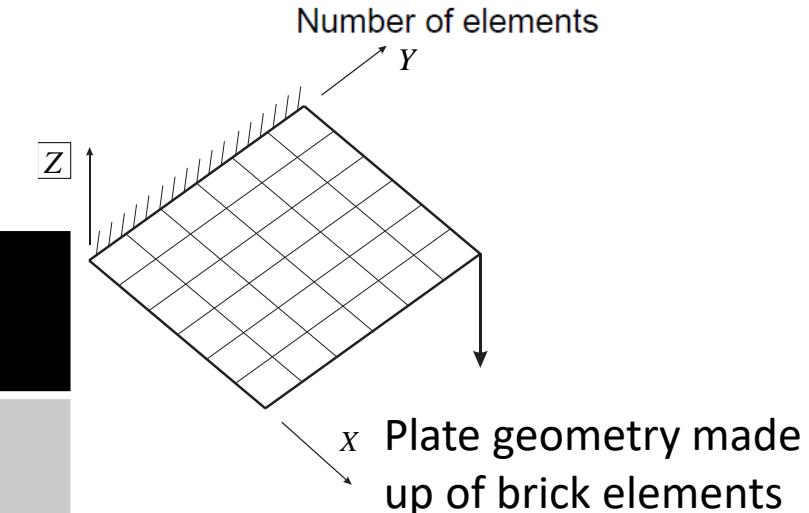
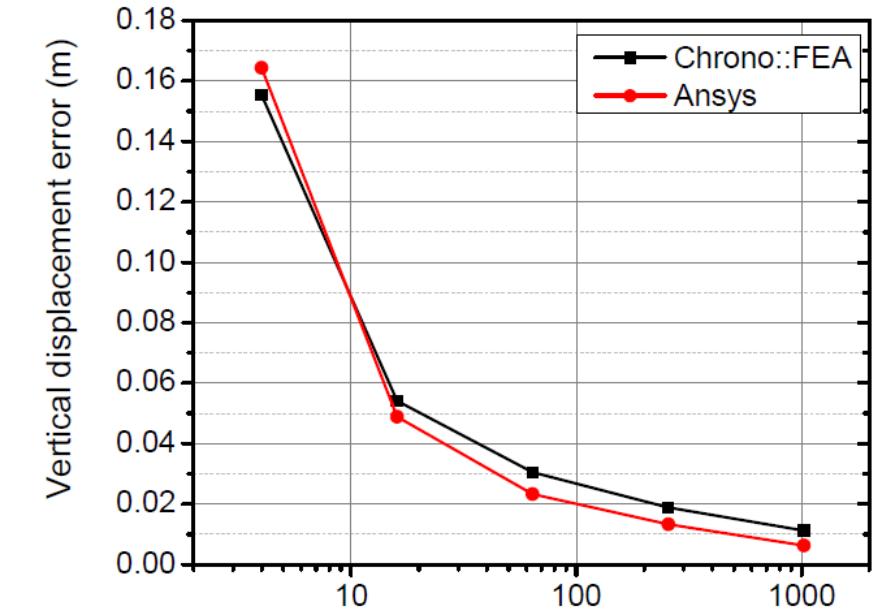
- Classical tri-linear element
- Implements Enhanced Assumed Strain formulation to alleviate locking
- Constitutive equations: Linear isotropic and Mooney-Rivlin

Dimensions	C_{10} (kPa)	C_{01} (kPa)	Vertical Force	Simulation type	Converged disp.
1mx1mx0.1m	50	10	-50N	Dynamic	-0.5762 m

Isotropic and MR

test_EASBrickIso.cpp

test_EASBrickMooneyR_Grav.cpp



Brick 9: Capped Drucker-Prager –Punch Test

Soil Material Properties

$$\sigma_{yield} = 210926 \text{ Pa}$$

$$\beta = 51.7848^\circ$$

$$\phi = 51.7848^\circ$$

$$R = 0.5$$

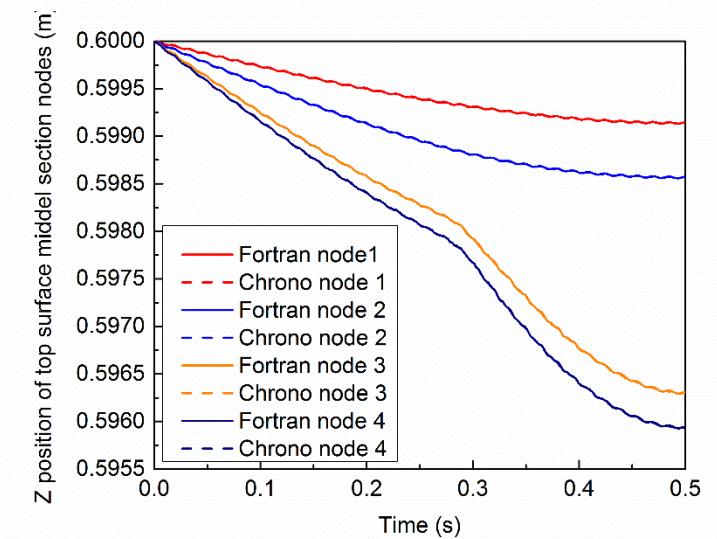
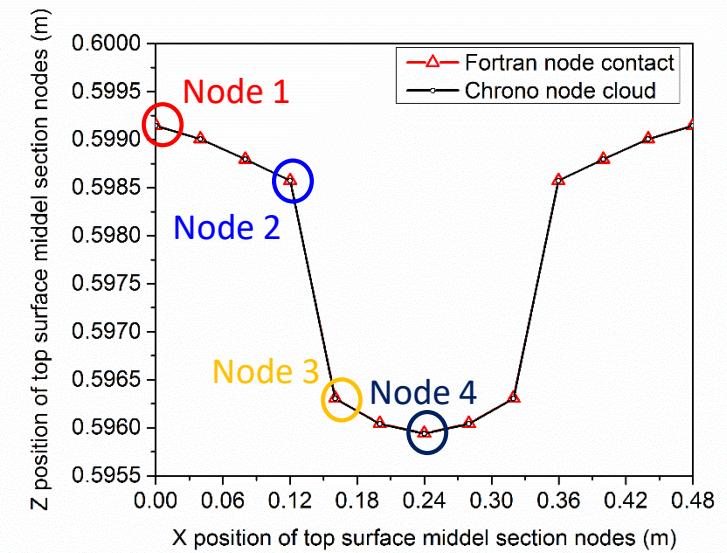
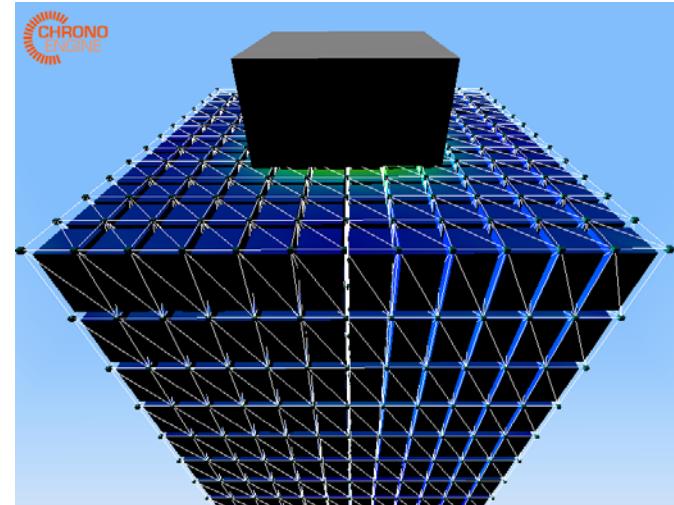
$$\rho = 2149 \text{ kg/m}^3$$

$$E = 54.1 \text{ MPa}$$

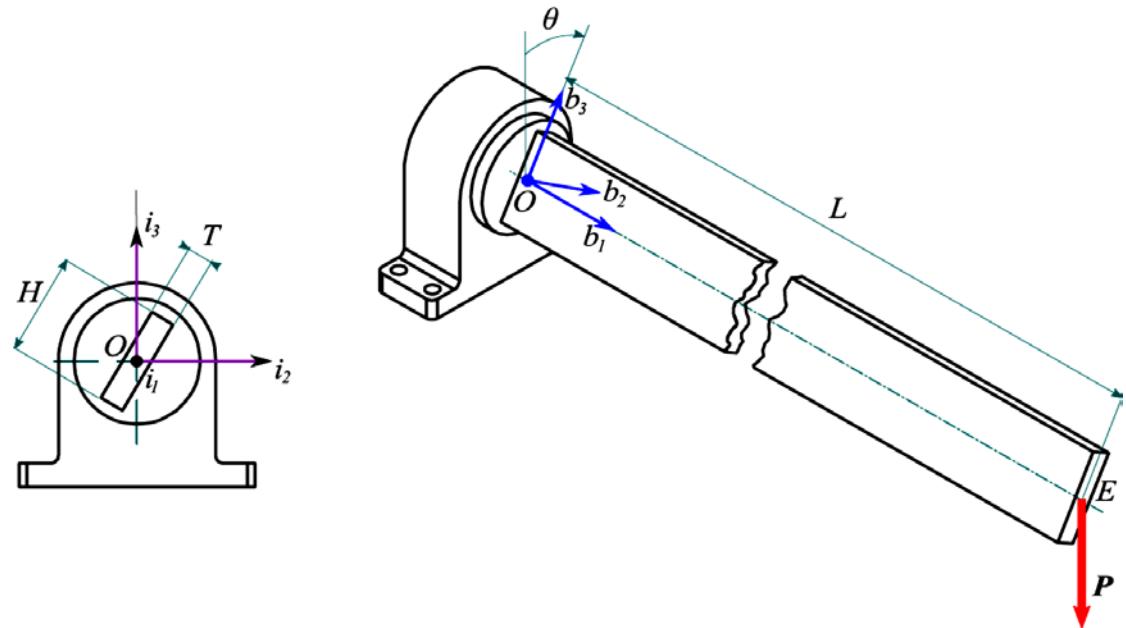
$$\nu = 0.293021$$

Chrono
verification
parameters

- Applied force : $-27000\sin(\pi t)$
- Contact stiffness : 165000 N/m
- Contact detection threshold : 0.009m
- Element number : 12*12*8
- Soil box dimension : 0.48m*0.48*0.6m
- Rigid punch dimension : 0.2m*0.2m*0.1m
- Bottom node fixed



Corotational Euler-Bernoulli beam: Princeton benchmark



$L = 0.508\text{m}$, $T = 3.2024\text{mm}$, $H = 12.77\text{mm}$,
Young modulus $E = 71.7\text{GPa}$, Poisson ratio = 0.31, $G = 27.37\text{GPa}$.

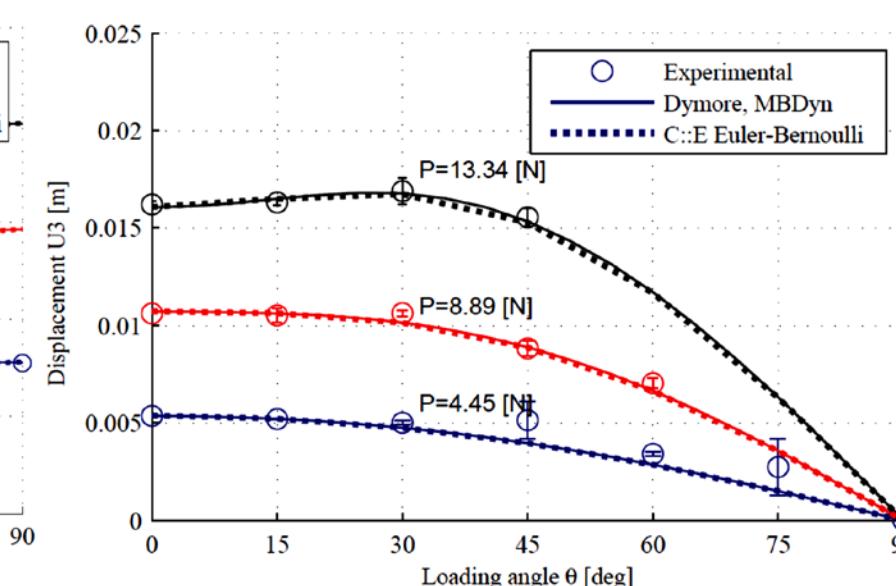
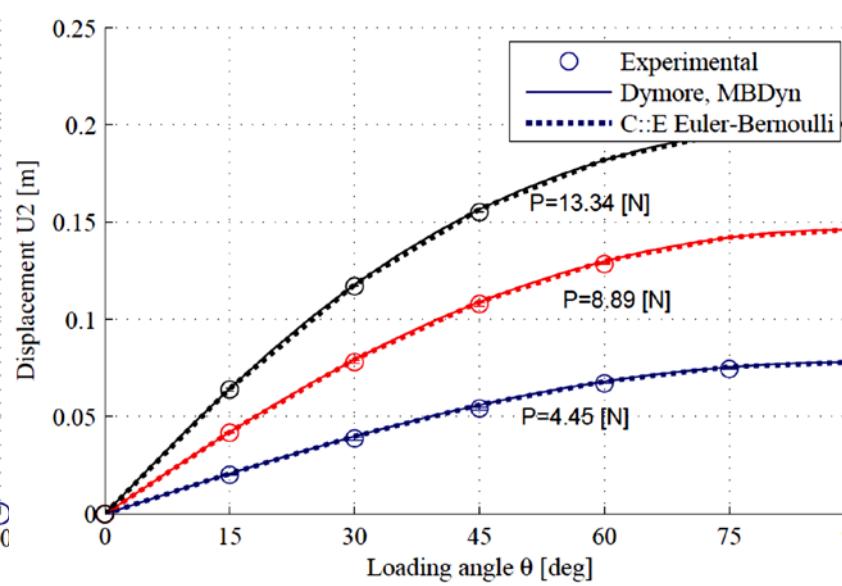
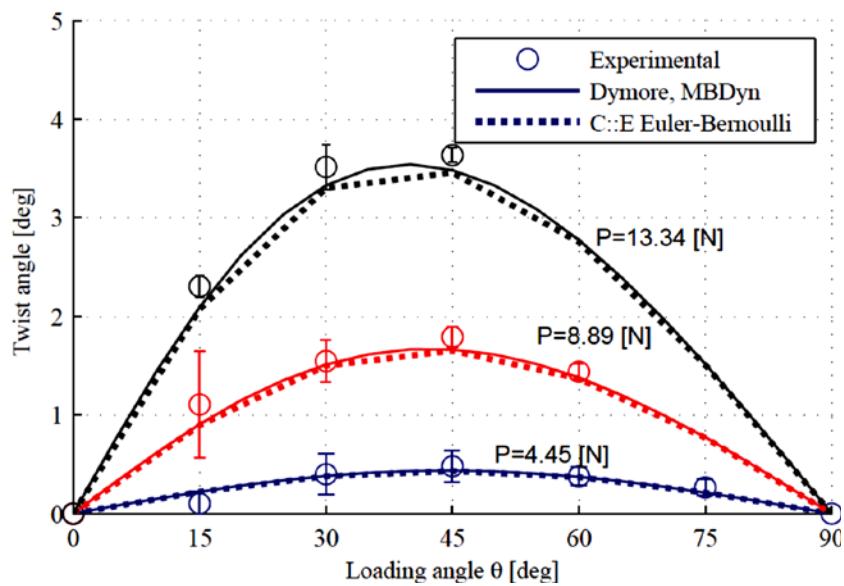
Three loading conditions are tested:

$P_1 = 4.448\text{N}$,

$P_2 = 8.896\text{N}$,

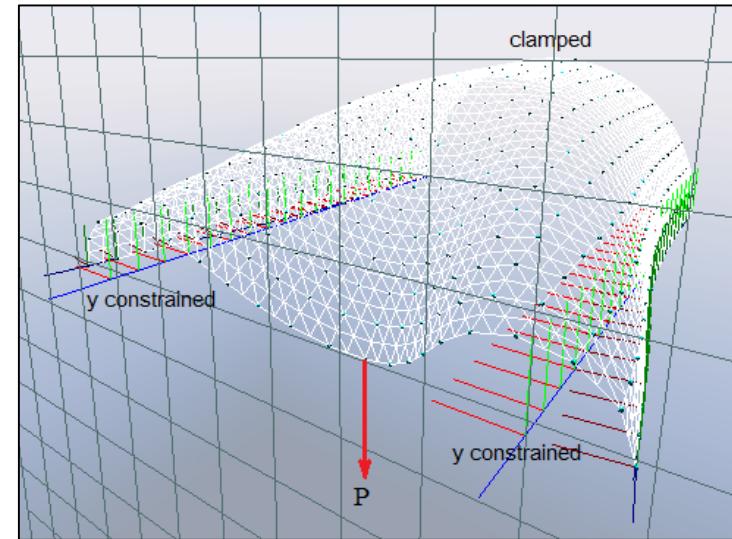
$P_3 = 13.345\text{N}$ for increasing values of the angle

More info: Tasora, A. "Validation of Euler-Bernoulli corotational beams in Chrono::Engine", Chrono white paper

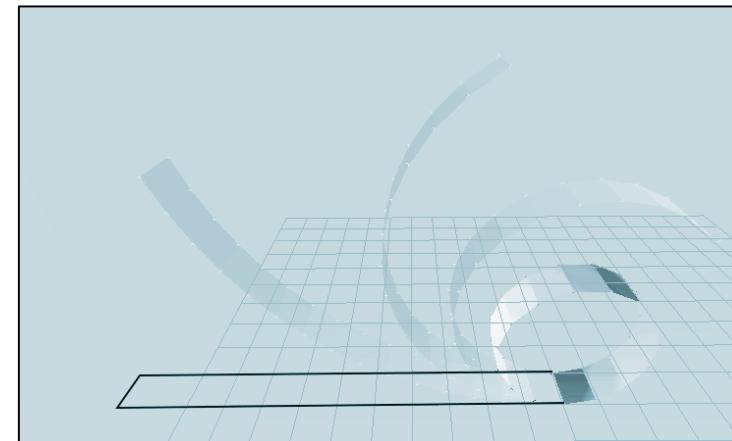


Kinematically exact Reissner shell element

Clamped half cylinder with sliding constraints at the sides



Large bending in a rolled band



*Comparison with results in literature
and with analytical solutions*

